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| PROJECT LENGTH                         | Prepared in<br><b>VAUGHN &amp;</b><br>1318-F P<br>ASHEVILL | the Office of:<br><b>MELTON</b><br>ATTON AVE.<br>E NC, 28806 |
|--|--|--|
|  | FOR THE NORTH CAROLIN                                      | NA DIVISION OF HIGHWAYS                                      |
| f TIP PROJECT 17BP.14.R.142 = 0.111 MI | 2012 STANDARD SPECIFICATIONS                               |  |
|  | LETTING DATE :   | HARDY WILLIS, PE<br>PROJECT ENGINEER                         |
|  |  | RYAN SHIPMAN, EI<br>Project design engineer                  |

| STATE STATE     | STATE PROJECT REFERENCE NO. |          |            |  |  |  |
|-----------------|-----------------------------|----------|------------|--|--|--|
| N.C. 17B        | P.14.R.142                  | •        |            |  |  |  |
| STATE PROJ. NO. | F. A. PROJ. NO.             | DESCRIPT | rion       |  |  |  |
| 17BP.14.R.142   | N⁄A                         | P.       | <b>E</b> . |  |  |  |
| 17BP.14.R.142   | N⁄A                         | R/W &    | UTIL.      |  |  |  |
| 17BP.14.R.142   | N⁄A                         | COM      | NST.       |  |  |  |
|                 |                             |          |            |  |  |  |
|                 |                             |          |            |  |  |  |
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|                 |                             |          |            |  |  |  |



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|--|--|--|--|--|--|--|--|--|--|
| 3  | NOTES:   |  |  |  |  |  |  |  |  |
|  | ASSUMED LIVE LOADHL-93 OR ALTERNATE LOADING  |  |  |  |  |  |  |  |  |
|  | DESIGN FILLMAX. = 4.090' MIN. = 3.942'   |  |  |  |  |  |  |  |  |
|  | FOR OTHER DESIGN DATA AND NOTES SEE STANDARD NOTE SH   | EET.   |  |  |  |  |  |  |  |
|  | 3″Ø WEEP HOLES INDICATED TO BE IN ACCORDANCE WITH THE SPECIFICATIONS.  |  |  |  |  |  |  |  |  |
|  | CONCRETE IN CULVERTS TO BE POURED IN THE FOLLOWING OF<br>"STAGING DIAGRAM", SHEET C-3):  | RDER (SEE ALSO   |  |  |  |  |  |  |  |
| WOODS  | 1.FOR STAGE 1A, WING FOOTINGS AND FLOOR SLAB INCLUD<br>OF ALL VERTICAL WALLS.  | ING 4"   |  |  |  |  |  |  |  |
|  | 2. THE REMAINING PORTIONS OF THE WALLS AND WINGS OF HEIGHT, FOLLOWED BY ROOF SLAB AND HEADWALLS.   | 2. THE REMAINING PORTIONS OF THE WALLS AND WINGS OF STAGE 1A,FULL<br>HEIGHT,FOLLOWED BY ROOF SLAB AND HEADWALLS. |  |  |  |  |  |  |  |
|  | 3. REPEAT SEQUENCE ABOVE FOR STAGES 1B, 2A, AND 2B.  |  |  |  |  |  |  |  |  |
| IS CLASS II RIP RAP<br>Pay item)   | THE RESIDENT ENGINEER SHALL CHECK THE LENGTH OF CULVER<br>STAKING IT OUT TO MAKE CERTAIN THAT IT WILL PROPERLY<br>OF THE FILL.   | RT BEFORE<br>Take care   |  |  |  |  |  |  |  |
| TION = 57 CY   | DIMENSIONS FOR WING LAYOUT AS WELL AS ADDITIONAL REI<br>EMBEDDED IN BARREL ARE SHOWN ON WING SHEET.  | NFORCING STEEL   |  |  |  |  |  |  |  |
| II RIP RAP<br>ENCH DETAIL<br>7 OF 10)<br>S CLASS II RIP RAP<br>PAY ITEM)<br>TION = 81 CY | AT THE CONTRACTOR'S OPTION, HE MAY SPLICE THE VERTICAL<br>IN THE INTERIOR FACE OF EXTERIOR WALL AND BOTH FACES<br>ABOVE LOWER WALL CONSTRUCTION JOINT. THE SPLICE LENGTH<br>IN THE SPLICE LENGTH CHART SHOWN ON THE PLANS. EXTRA W<br>TO THE SPLICES SHALL BE PAID FOR BY THE CONTRACTOR.<br>NO PRECAST REINFORCED BOX CULVERT OPTION WILL BE ALLO<br>FOR CULVERT DIVERSION DETAILS AND PAY ITEMS, SEE EROST<br><u>GRADE DATA</u><br>GRADE POINT ELEV. @ STA. 13+47.70 -L- = 1013.56 ±<br>BED ELEV. @ 13+47.70 -L- = 1001.54 ± | REINFORCING STEEL<br>OF INTERIOR WALLS<br>SHALL BE AS PROVIDED<br>GEIGHT OF STEEL DUE                            |  |  |  |  |  |  |  |
| <u> </u><br>) – <u>L</u> –   | ROADWAY SLOPES 2:1   |  |  |  |  |  |  |  |  |
|  | HYDRAULIC DATA   | TOTAL STRUCTU  |  |  |  |  |  |  |  |
|  | DESIGN DISCHARGE = 650 CFS<br>DESIGN FREQUENCY = 25 YRS<br>DESIGN HW ELEVATION = 1008.30 FT<br>BASE DISCHARGE = 950 CFS<br>BASE FREQUENCY = 100 YRS<br>BASE HW ELEVATION = 1009.30 FT<br>DRAINAGE AREA = 1.92 SO.MI.<br>W.S. ELEVATION = 1004.20 FT  | CLASS A CONCRETE<br>BARREL @ <u>1.90</u> CY<br>WINGS ETC. <u>32.1</u><br>TOTAL <u>127.3</u>                      |  |  |  |  |  |  |  |
|  | AT DATE OF SURVEY<br>(4/17/2013)   | BARREL 15  |  |  |  |  |  |  |  |
|  | OVERTOPPING FLOOD DATA   | TOTAL17  |  |  |  |  |  |  |  |
|  | OVERTOPPING DISCHARGE = 1530 CFS<br>OVERTOPPING FREQUENCY = 500+ YRS<br>OVERTOPPING ELEVATION = 1011.00 FT   | CULVERT EXCAVATION<br>FOUNDATION CONDITION<br>MATERIAL   |  |  |  |  |  |  |  |
|  |  | DEMOVAL OF EXISTING  |  |  |  |  |  |  |  |

REMOVAL OF EXISTING STRUCTURE CHANNEL SUBSTRATE MA RIP-RAP, CLASS II ASBESTOS ASSESSMENT



EXCAVATE ONE FOOT BELOW THE BOTTOM OF CULVERT AND WING FOOTING ELEVATIONS.REPLACE WITH FOUNDATION CONDITIONING MATERIAL IN ACCORDANCE WITH ARTICLE 414-4 OF THE STANDARD SPECIFICATIONS.

FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

AFTER SERVING AS A TEMPORARY STRUCTURE THE EXISTING STRUCTURE CONSISTING OF A SINGLE SPAN, 26.0 FOOT LONG AND 18'-7<sup>/</sup><sub>16</sub>" WIDE BRIDGE WITH TIMBER FLOOR ON STEEL I-BEAMS ON TIMBER ABUTMENTS AND LOCATED AT THE PROPOSE STRUCTURE SHALL BE REMOVED. THE EXISTING BRIDGE IS PRESENTLY POSTED FOR LOAD LIMIT. SHOULD THE STRUCTURAL INTEGRITY OF THE BRIDGE DETERIORATE DURING CONSTRUCTION OF THE PROPOSED BRIDGE, A LOAD LIMIT MAY BE POSTED AND MAY BE REDUCED AS FOUND NECESSARY DURING THE LIFE OF THE PROJECT.

REMOVAL OF THE EXISTING BRIDGE SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL INTO THE WATER. THE CONTRACTOR SHALL REMOVE THE BRIDGE AND SUBMIT PLANS FOR DEMOLITION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS.

INASMUCH AS THE PAINT SYSTEM ON THE EXISTING STRUCTURAL STEEL CONTAINS LEAD, THE CONTRACTOR'S ATTENTION IS DIRECTED TO ARTICLE 107-1 OF THE STANDARD SPECIFICATIONS, ANY COSTS RESULTING FROM COMPLIANCE WITH APPLICABLE STATE OR FEDERAL REGULATIONS PERTAINING TO HANDLING OF MATERIALS CONTAINING LEAD BASED PAINT SHALL BE INCLUDED IN THE BID PRICE FOR 'REMOVAL OF EXISTING STRUCTURE AT STATION 13+47.70 -L-.

A 3 FOOT STRIP OF FILTER FABRIC SHALL BE ATTACHED TO THE FILL FACE OF THE WING COVERING THE ENTIRE LENGTH OF THE EXPANSION JOINT.

FOR ASBESTOS ASSESSMENT FOR BRIDGE DEMOLITION AND RENOVATION ACTIVITIES, SEE SPECIAL PROVISIONS.

|  | l           | HORTZ, CURVE DATA                    |                        |
|--|-------------|--------------------------------------|------------------------|
| RE QUANTITIES  |             | PT_STA - 14+65 33                    | -                      |
|  |             | $\Delta = 6^{\circ} 17' 24.3'' (LT)$ |                        |
|  |             | $D = 4^{\circ} 53' 49.5''$           |                        |
| Y/FI <u> </u>  |             | T = 64.29'                           |                        |
| C.Y.   |             | R = 1,170.00'                        |                        |
| C.Y.   |             | RO = 34.5                            |                        |
|  |             |                                      |                        |
| 5,162 LBS.   |             |                                      |                        |
| LBS.   |             |                                      |                        |
| 280 I BS.  |             |                                      |                        |
| <u>,                                    </u>                       |             |                                      |                        |
| LUMP SUM   |             |                                      |                        |
| NTNG 65 TONS   |             |                                      |                        |
| <u> </u>   |             |                                      |                        |
| LUMP SUM   |             |                                      |                        |
|  |             |                                      |                        |
| ATERIAL <u>/2</u> IONS   |             |                                      |                        |
| <u>   127   </u> TONS  |             |                                      |                        |
| LUMP SUM   | Г           | оронсат No 178Р 14 R                 | 142                    |
|  |             | RUJECI NU. <u>IIDI III</u>           |                        |
|  |             | POLK (                               | COUNTY                 |
|  |             |                                      |                        |
|  |             | STATION: <u>13+47.70</u> -L-         |                        |
|  |             | SHEET 1 OF 10 REPLACE BR             | IDGE No.189            |
|  | ſ           |                                      |                        |
|  |             | STATE OF NORTH CAROLINA              |                        |
| 423-467-840  | 1           | RALEIGH                              | ATION                  |
| 865·546·580  | 00<br>a. SC |                                      |                        |
| onsulting Engineers<br>Charleston,                                 | 5<br>SC     | DOUBLE 8 FI.X 8                      |                        |
| Asheville, 843.974.565<br>North Carolina Middlesboro               | О<br>, КҮ   | CONCRETE BOX CU                      | I VFRT                 |
| 828-253-2796 606-248-660<br>narlotte, NC 🗆 Boone, NC 🗌 Atlanta, GA | 00          | 105° Skew                            | / / /                  |
| 14·357·0488 828·355·9933 770·627·350                               | 99          | IOJ JNLW                             |                        |
|  |             |                                      |                        |
| RWW DATE:3   | /2015       | REVISIONS                            | SHEET NO.              |
| BY: HLW DATE: 3  | /2015       | NO. BY: DATE: NO. BY: DATE:          | C - 1                  |
| DF RECORD: RTS DATE: 3   | /2015       | 1 3<br>9 4                           | TOTAL<br>SHEETS<br>1 ∩ |
|  |             | <u>《</u> 《 】                         |                        |
|  |             | STD.NO.CB222A                        |                        |





| D 8-2  | ASSEMBLED BY :          | R.W.W.           | DATE : <u>MAR. 2015</u> | SPECIAL  |
|--------|-------------------------|------------------|-------------------------|----------|
| WN II- | CHECKED BY :            | H.L.W.           | DATE : <u>MAR. 2015</u> |          |
| REVISE | DRAWN BY : <u>W.</u> E  | BRYAN STANLEY II | DATE : <u>NOV.1971</u>  | STANDARD |
| REDRA  | Checked by : <u>J</u> O | DEL A.JOHNSON    | DATE : <u>DEC.1971</u>  |          |

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BY M.M. CHECKED BY R.W.W. BY E.L.R. CHECKED BY G.R.F Y A.R.B. CHECKED BY C.R.K.

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# CONNECTION OF WING FOOTING AND FLOOR SLAB WHEN WING FOOTING IS THICKER THAN SLAB

DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



STD.NO.CB222



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| Docusigned by<br>HardyWilless, L. WILL<br>CC287FCF0223461.   | 28/2017   | PROJECT NO. <u>17BP.14.R.142</u><br><u>POLK</u> COUNTY<br>STATION: <u>13+47.70</u> -L-<br>SHEET 4 OF 10         |
|--|---|---|
| Consulting Engineers<br>Asheville,<br>North Carolina<br>828:253:2796<br>Charlotte, NC Boone, NC<br>704:357:0488 828:355:9933<br>pyright © 2006 Vaughn & Melton, Inc. | <ul> <li>Tr1-Cities, TN<br/>423·467·8401</li> <li>Knoxville, TN<br/>865·546·5800</li> <li>Spartanburg, SC<br/>864·574·4775</li> <li>Charleston, SC<br/>843·974·5650</li> <li>Middlesboro, KY<br/>606·248·6600</li> <li>Atlanta, GA<br/>770·627·3509</li> <li>All Rights Reserved</li> </ul> | DEPARTMENT OF TRANSPORTATION<br>RALEIGH<br>DOUBLE 8 FT. X 8 FT.<br>CONCRETE BOX CULVERT<br>105° SKEW<br>STAGE 2 |
| Y:RWW<br>BY: HLW<br>OF RECORD:RTS  | DATE: 3/2015<br>DATE: 3/2015<br>DATE: 3/2015  | REVISIONSSHEET NO.NO.BY:DATE:NO.BY:DATE:C-4135410   |



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DRAWN B CHECKED DES.EGR.

| DOCUMENT NOT CONSIDERED<br>FINAL UNLESS ALL<br>SIGNATURES COMPLETED  |   |
|--|---|
| Docusigned by Dy L. WILLING AV28/2017  | PROJECT NO. <u>17BP.14.R.142</u><br><u>POLK</u> COUNTY<br>STATION: <u>13+47.70 -L-</u><br>SHEET 5 OF 10         |
| CC287FCF0223461         Image: Construction of the state of th | DEPARTMENT OF TRANSPORTATION<br>RALEIGH<br>DOUBLE 8 FT. X 8 FT.<br>CONCRETE BOX CULVERT<br>105° SKEW<br>STAGE 1 |
| BY: RWW DATE: 3/20   | 5 REVISIONS SHEET NO.   |
| BY: HLW DATE: 3/20<br>. OF RECORD: RTS DATE: 3/20  | 5     NO.     BT:     DATE:     NO.     BY:     DATE:     COS       5     1     3     TOTAL     SHEETS     10   |



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| Docusigned by POF  | Summer Sum | PROJEC<br>STATION<br>SHEET 6 0 | T NO<br>POLK<br>N:                               | 17BP.<br>-47.70  | <u>14.R.1</u><br>CO<br>_L- | 42<br>UNTY                                |
|--|---|--------------------------------|--|--|----------------------------|---|
| Consulting Engineers<br>Asheville,<br>North Carolina<br>828.253.2796<br>Charlotte, NC Boone, NC<br>704.357.0488 828.355.9933<br>pyright © 2006 Vaughn & Melton, Inc. | <ul> <li>Tri-Cities, TN<br/>423-467-8401</li> <li>Knoxville, TN<br/>865-546-5800</li> <li>Spartanburg, SC<br/>864-574-4775</li> <li>Charleston, SC<br/>843-974-5650</li> <li>Middlesboro, KY<br/>606-248-6600</li> <li>Atlanta, CA<br/>770-627-3509</li> <li>All Rights Reserved</li> </ul>   | depaf<br>DOU<br>CONC           | state<br>RTMENT 0<br>IBLE 8<br>RETE<br>105<br>ST | of north carol<br>F TRANS<br>Raleigh<br>BOX<br>° SK<br>AGE 2 | X 8<br>CUL                 | fion<br>FT.<br>VERT                       |
| Y:RWW<br>BY: HLW<br>OF RECORD:RTS  | DATE: 3/2015<br>DATE: 3/2015<br>DATE: 3/2015  | NO. BY:                        | REVISIOI<br>DATE: NO<br>3<br>4                   | NS<br>BY:  | DATE:                      | SHEET NO.<br>C-6<br>total<br>sheets<br>10 |





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NATIVE MATERIAL CONSISTS OF MATERIAL THAT IS EXCAVATED FROM THE STREAM BED OF FLOODPLAIN AT THE PROJECT SITE DURING CULVERT CONSTRUCTION. ONLY MATERIAL THAT IS EXCAVATED FROM THE STREAM BED MAY BE USED TO LINE THE LOW FLOW CULVERT BARREL.RIP RAP MAY BE USED TO SUPPLEMENT THE NATIVE MATERIAL IN THE HIGH FLOW CULVERT BARREL. IF RIP RAP IS USED TO LINE THE HIGH FLOW CULVERT BARREL.NATIVE MATERIAL SHOULD BE PLACED ON TOP TO FILL VOIDS AND PROVIDE A FLAT SURFACE FOR ANIMAL PASSAGE.NATIVE MATERIAL IS SUBJECT TO APPROVAL BY THE ENGINEER AND MAY BE SUBJECT TO PERMIT CONDITIONS.

THE STOCKPILED NATIVE MATERIAL SHALL BE PLACED AS SHOWN IN THE "PLAN OF FLOOR SILL LAYOUT "SKETCH TO PROVIDE A 1'-O"DEPTH LOW FLOW CHANNEL BETWEEN LOW FLOW SILLS, AND SHALL BE PLACED TO A DEPTH OF 2'-O" BETWEEN HIGH FLOW SILLS.

SUPPLEMENTAL STONE OF SIMILAR CHARACTERISTIC OF THE NATIVE MATERIAL MAY BE USED AS NECESSARY WITH APPROVAL BY ENGINEER.

THE ENTIRE COST OF WORK REQUIRED TO PLACE THE EXCAVATED MATERIAL SHALL BE INCLUDED IN THE CONTRACT PRICE BID FOR CHANNEL SUBSTRATE MATERIAL.

THE ENTIRE COST OF WORK REQUIRED TO CONSTRUCT THE SILLS SHALL BE INCLUDED IN THE VARIOUS PAY ITEMS. TOP OF LOW FLOW SILLS SHOULD MATCH STREAM BED ELEVATION IN LOW FLOW CHANNEL OF STREAM. (THALWEG) DO NOT SET ELEVATION OF HIGH SILL ABOVE BANK FILL. NUMBER OF SILLS DETERMINED BY THE ENGINEER.

# NOTES

| E          |          |          |        |         |   |            |         |                |         |       | RIA    |             | OF      | BILL     |         |         |        |        |   |          |        |         |         |       |
|------------|----------|----------|--------|---------|---|------------|---------|----------------|---------|-------|--------|-------------|---------|----------|---------|---------|--------|--------|---|----------|--------|---------|---------|-------|
| _          |          | 2B       | GE á   | STA     |   |            |         | Д              | = 2     | ΤA(   |        |             |         | 1B       | GE :    | STA     |        |        |   | 1 A      | GE :   | STA     |         |       |
| VE         | WEIGHT   | LENGTH   | TYPE   | SIZE    | NO.   | BAR        | WEIGHT  | ENGTH          | YPE     | SIZE  | NO.    | BAR         | WEIGHT  | LENGTH   | TYPE    | SIZE    | NO.    | BAR    | WEIGHT                                  | LENGTH   | TYPE   | SIZE    | NO.     | BAR   |
|            | 216      | 5'-7"    |        | 4       | 58  | A1         | 216     | 5'-7"          | 1       | 4     | 58     | A1          | 224     | 5'-7"    | 1       | 4       | 60     | A1     | 209                                     | 5′-7″    | 1      | 4       | 56      | A1    |
|            | 541      | 6′-8″    | STR.   | 6       | 54  | A700       | 898     | 11'-6"         | TR.     | 6     | 52     | A500        | 601     | 6'-8"    | STR.    | 6       | 60     | A300   | 898                                     | 11'-6"   | STR.   | 6       | 52      | A100  |
|            | 23       | 7'-6"    | STR.   | 6       | 2   | A701       | 27      | 8'-10"         | TR.     | 6     | 2      | A501        | 11      | 3'-6"    | STR.    | 6       | 2      | A301   | 36                                      | 11'-11"  | STR.   | 6       | 2       | A101  |
|            | 15       | 4'-11"   | STR.   | 6       | 2   | A702       | 14      | 2'-3"          | TR.     | 6     | 4      | A502        |         |          |         |         |        |        | 27                                      | 8'-10"   | STR.   | 6       | 2       | A102  |
| [          | 5        | 1'-9"    | STR.   | 6       | 2   | A703       | 32      | 10'-8"         | TR.     | 6     | 2      | A503        |         |          |         |         |        |        | 17                                      | 5′-8″    | STR.   | 6       | 2       | A103  |
|            |          |          |        |         |   |            | 23      | 7'-7″          | TR.     | 6     | 2      | A504        |         |          |         |         |        |        | 8                                       | 2'-7"    | STR.   | 6       | 2       | A104  |
|            |          |          |        |         |   |            | 13      | 4'-5"          | TR.     | 6     | 2      | A505        |         |          |         |         |        |        | 55                                      | 9'-2"    | STR.   | 6       | 4       | A105  |
|            | 541      | 6'-8"    | STR    | 6       | 54  | <u> </u>   | 898     | 11'-6"         | TR      | 6     | 52     | <u>A600</u> | 601     | 6'-8"    | STR.    | 6       | 60     | A400   | 898                                     | 11'-6"   | STR.   | 6       | 52      | A200  |
|            | 23       | 7'-6"    | STR.   | 6       | 2   | A801       | 27      | 8'-10"         | TR.     | 6     | 2      | A601        | 11      | 3'-6"    | STR.    | 6       | 2      | A401   | 36                                      | 11'-11"  | STR.   | 6       | 2       | A201  |
|            | 15       | 4'-11"   | STR.   | 6       | 2   | A802       | 14      | 2'-3"          | TR.     | 6     | 4      | A602        |         | 5 0      | 0 11 1  |         |        | ////01 | 27                                      | 8'-10"   | STR.   | 6       | 2       | A202  |
| BAR DIMENS | 5        | 1'-9"    | STR.   | 6       | 2   | A803       | 32      | 10'-8"         | TR.     | 6     | 2      | A603        |         |          |         |         |        |        | 17                                      | 5'-8"    | STR.   | 6       | 2       | A203  |
|            |          |          |        |         |   |            | 23      | 7'-7"          | TR.     | 6     | 2      | A604        |         |          |         |         |        |        | 8                                       | 2'-7"    | STR.   | 6       | 2       | A204  |
| SPLIC      |          |          |        |         |   |            | 13      | 4'-5"          | TR.     | 6     | 2      | A605        |         |          |         |         |        |        | 55                                      | 9'-2"    | STR.   | 6       | 4       | A205  |
| BAR S      |          | 01 7/    | CTD    | 4       | 6.0   | <b>D</b> 1 | 7 4 1   | 0/ 7//         | <u></u> | 4     | 100    | <b>D</b> 1  | 771     | 0/ 7//   | CTD     | 4       | 60     |        | 7 4 1                                   | 01.7%    | CTD    | 4       | 100     |       |
| #4         | 571      | 9'-3"    | SIR.   | 4       | 60  | BI         | (4]     | 9'-3"          | IK.     | 4     | 120    | BI          | 571     | 9'-3"    | SIR.    | 4       | 60     | BI     | (4]                                     | 9'-3"    | SIR.   | 4       | 120     | BI    |
| #5         | 824      | 24'-8"   | STR.   | 4       | 50  | C2         | 1285    | 24'-8"         | TR.     | 4     | 78     | C2          | 893     | 26'-9"   | STR.    | 4       | 50     | C1     | 1394                                    | 26'-9"   | STR.   | 4       | 78      | C1    |
| #6         |          |          |        |         |   |            |         |                |         |       |        |             |         |          |         |         |        |        |   |          |        |         |         |       |
| TOTAL REIN | 6        | 1'-5"    | STR.   | 6       | 3   | D2         | 11      | 2'-5″          | TR.     | 6     | 3      | D1          | 13      | 1'-5″    | STR.    | 6       | 6      | D2     | 22                                      | 2'-5"    | STR.   | 6       | 6       | D1    |
|            | 29       | 6'-11"   | STR.   | 5       | <u>ــــــــــــــــــــــــــــــــــــ</u> | 62         | 49      | 1'-10"         | TR.     | 5     | Δ      | G1          | 58      | 6'-11"   | STR.    | 5       | 8      | 62     | 99                                      | 11'-10"  | STR.   | 5       | 8       | G1    |
| COLVEN     | 23       |          |        |         |   | 02         | 1.5     |                |         |       | •      | 01          | 30      | 0 11     | 0 1 1 1 |         | 0      | UL     | 55                                      | 11 10    | 0.11.1 | 5       | 0       | 01    |
| SILLS      | 111      | 6'-11"   | STR.   | 8       | 6   | S3         | 190     | 11'-10″        | TR.     | 8     | 6      | S2          | 222     | 6'-11"   | STR.    | 8       | 12     | S3     | 379                                     | 11'-10"  | STR.   | 8       | 12      | S2    |
|            | 2725 I B | F 2B)• 2 | (STAG  | G STEFI |   | RETNE      | 506 I B | 2A) <b>□</b> 4 | STAGE   | STEFI |        | RETNE       | 005   B | - 1R)• 3 | (STAG   | G STEFI |        | RETNE  | 926 I B                                 | F 1Λ)• 4 | (STAG  | STEFI   |         | RETNE |
|            |          | 2B)      | (STAGE | NCRETE  | S A CO                                      | CLASS      | ,       | A)             | TAGE    | RETE  | A CON  | CLASS       | ,       | 1B)      | (STAGE  | NCRETE  | S A CO | CLASS  | ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,, | 1A)      | (STAGE | NCRETE  | S A CON | CLASS |
|            |          | 3.4 C.Y. | EL 18  | t barr  | CULVER                                      |            |         | C.Y.           | 29.     | BARRE | ULVERT | C           |         | .4 C.Y.  | EL 18   | t barr  | CULVER |        |   | 0.2 C.Y. | EL 29  | r barre | CULVER  | (     |
|            |          | 0.3 C.Y. | (      |         | SILLS                                       |            |         | C.Y.           | 0.      |       | ILLS   | S           |         | .6 C.Y.  | C       |         | SILLS  |        |   | .2 C.Y.  | 1      |         | SILLS   |       |

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#### DOCUMENT NOT CONSIDERED FINAL UNLESS ALL SIGNATURES COMPLETED



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|   |   | PROJEC              | T NO.                                  | _17BP.  | .14.R.14                        | 12                  |
|---|---|---------------------|--|---|---------------------------------|---------------------|
|   |   | P                   | OLK                                    |   | CO                              | UNTY                |
|   |   | STATIO              | )N:                                    | 13+47   | 2.70 -L                         |                     |
|   |   | SHEET 8 C           | )F 10                                  |   |                                 |                     |
| Vaughn & Melton<br>Consulting Engineers<br>Asheville,<br>North Carolina<br>828.253.2796<br>Charlotte, NC Boone, NC<br>704.357.0488 828.355.9933<br>Copyright © 2006 Vaughn & Melton, Inc. | <ul> <li>Tri-Cities, TN<br/>423-467-8401</li> <li>Knoxville, TN<br/>865-546-5800</li> <li>Sportanburg, SC<br/>864-574-4775</li> <li>Charleston, SC<br/>843-974-5650</li> <li>Middlesboro, KY<br/>606-248-6600</li> <li>Atlanta, GA<br/>770-627-3509</li> <li>All Rights Reserved</li> </ul> | depa<br>DOL<br>CONC | stati<br>RTMENT<br>JBLE<br>CRETE<br>10 | e of north card<br>OF TRAN<br>RALEIGH<br>8 FT<br>5 ° SI | NSPORTA<br>SPORTA<br>CUL<br>KEW | tion<br>FT.<br>VERT |
| N BY:RWW  | DATE: 3/2015  |                     | REVIS                                  | SIONS   |                                 | SHEET NO.           |
| ED BY: HLW  | DATE: 3/2015  | NO. BY:             | DATE:                                  | NO. BY:   | DATE:                           |                     |
| GR.OF RECORD:RTS  | DATE: 3/2015  | 12                  |  | :জ<br>ব্রু  |                                 | SHEETS<br>10        |



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SECTION



STD.NO.CW7508

|        |   |   | LO             | AD A                 | AND R                | ESIS       |                                   | CEF        | ACTO | R RATIN       | G (L  | RFR)       | те  |                |                                     |              |
|--------|---|---|----------------|----------------------|----------------------|------------|-----------------------------------|------------|------|---------------|---|------------|-----|----------------|-------------------------------------|--------------|
|        |   |   | SUMM           | ARY                  | FOR                  | RETN       | FORC                              | ED (       | CONC | KFIF RO>      | K CU  | LVER       | 15  |                |                                     |              |
|        |   |   |                |                      |                      |            | STRENGTH I LIMIT STATE            |            |      |               |   |            |     |                |                                     |              |
|        |   |   |                |                      |                      |            |                                   |            |      | MOMENT        |   |            |     | SHEAR          |                                     | 1            |
|        |   |   |                |                      |                      |            |                                   |            |      |               |   |            |     |                |                                     |              |
|        |   | ICLE                                    | GHT (W)<br>VS) | TROLLING<br>D RATING | IIMUM<br>ING FACTORS | S = W × RF | E-LOAD<br>Tors (γ <sub>LL</sub> ) | ING FACTOR | °ON  | MENT          | TANCE FROM<br>T END OF<br>MENT (f+)   | ING FACTOR | "ON | E M<br>E       | TANCE FROM<br>T END OF<br>MENT (f+) | IMENT NUMBER |
| ΓEΛ    |   | E H H H H H H H H H H H H H H H H H H H | WE T<br>(TON   | CON                  | MIN<br>RAT<br>(RF)   | TON        | LIV<br>FAC                        | RAT        | BOX  | ELE           | D I S<br>L E L<br>E L<br>E L<br>E L<br>E L<br>E L<br>E L<br>E L<br>E L<br>E | RAT        | BOX | L A P<br>T A P | D I S<br>L E F<br>E L E             | CON          |
|        |   | HL-93 (INVENTORY)                       | N/A            | $\langle 1 \rangle$  | 1.57                 |            | 1.75                              | 1.81       | 1    | EXTERIOR WALL | 8.33  | 1.57       | 1   | BOTTOM SLAB    | 7.82                                |              |
| DESIGN |   | HL-93 (OPERATING)                       | N/A            |                      | 2.04                 |            | 1.35                              | 2.35       | 1    | EXTERIOR WALL | 8.33  | 2.04       | 1   | BOTTOM SLAB    | 7.82                                |              |
| RATING |   | HS-20 (INVENTORY)                       | 36.000         | $\langle 2 \rangle$  | 1.57                 | 56.52      | 1.75                              | 1.85       | 1    | BOTTOM SLAB   | 8.42  | 1.57       | 1   | BOTTOM SLAB    | 7.82                                | •            |
|        |   | HS-20 (OPERATING)                       | 36.000         | •                    | 2.04                 | 73.44      | 1.35                              | 2.40       | 1    | BOTTOM SLAB   | 8.42  | 2.04       | 1   | TOP SLAB       | 7.82                                | •            |
|        |   | SNSH                                    | 13.500         |                      | 2.97                 | 40.10      | 1.40                              | 2.97       | 1    | EXTERIOR WALL | 8.33  | 2.97       | 1   | TOP SLAB       | 7.82                                |              |
|        |   | SNGARBS2                                | 20.000         |                      | 2.76                 | 55.20      | 1.40                              | 2.84       | 1    | EXTERIOR WALL | 8.33  | 2.76       | 1   | TOP SLAB       | 7.82                                |              |
|        | ICLE  | SNAGRIS2                                | 22.000         |                      | 2.87                 | 63.14      | 1.40                              | 2.97       | 1    | EXTERIOR WALL | 8.33  | 2.87       | 1   | BOTTOM SLAB    | 7.82                                |              |
|        | <pre> </pre> | SNCOTTS3                                | 27.250         |                      | 2.23                 | 60.77      | 1.40                              | 2.74       | 1    | BOTTOM SLAB   | 8.42  | 2.23       | 1   | BOTTOM SLAB    | 7.82                                |              |
|        | SLE<br>(S   | SNAGGRS4                                | 34.925         |                      | 1.90                 | 66.36      | 1.40                              | 2.35       | 1    | BOTTOM SLAB   | 8.42  | 1.90       | 1   | BOTTOM SLAB    | 7.82                                |              |
|        | INC   | SNS5A                                   | 35.550         |                      | 2.21                 | 78.57      | 1.40                              | 2.73       | 1    | BOTTOM SLAB   | 8.42  | 2.21       | 1   | BOTTOM SLAB    | 7.82                                |              |
|        |   | SNS6A                                   | 39.950         |                      | 2.26                 | 90.29      | 1.40                              | 2.79       | 1    | BOTTOM SLAB   | 8.42  | 2.26       | 1   | BOTTOM SLAB    | 7.82                                |              |
| LEGAL  |   | SNS7B                                   | 42.000         |                      | 2.43                 | 102.06     | 1.40                              | 2.99       | 1    | BOTTOM SLAB   | 8.42  | 2.43       | 1   | BOTTOM SLAB    | 7.82                                |              |
| RATING | ER  | TNAGRIT3                                | 33.000         |                      | 2.30                 | 75.90      | 1.40                              | 2.82       | 1    | BOTTOM SLAB   | 8.42  | 2.30       | 1   | BOTTOM SLAB    | 7.82                                |              |
|        | RAI   | TNT4A                                   | 33.075         |                      | 2.47                 | 81.70      | 1.40                              | 3.04       | 1    | BOTTOM SLAB   | 8.42  | 2.47       | 1   | BOTTOM SLAB    | 7.82                                |              |
|        | L-IN  | TNT6A                                   | 41.600         |                      | 2.52                 | 104.83     | 1.40                              | 3.10       | 1    | BOTTOM SLAB   | 8.42  | 2.52       | 1   | BOTTOM SLAB    | 7.82                                |              |
|        | SEN<br>ST)  | TNT7A                                   | 42.000         |                      | 2.77                 | 116.34     | 1.40                              | 3.41       | 1    | BOTTOM SLAB   | 8.42  | 2.77       | 1   | BOTTOM SLAB    | 7.82                                | •            |
|        | CTOR<br>(TT   | TNT7B                                   | 42.000         |                      | 2.77                 | 116.34     | 1.40                              | 3.41       | 1    | BOTTOM SLAB   | 8.42  | 2.77       | 1   | BOTTOM SLAB    | 7.82                                |              |
|        | TRA(  | TNAGRIT4                                | 43.000         | $\langle 3 \rangle$  | 1.90                 | 81.70      | 1.40                              | 2.34       | 1    | BOTTOM SLAB   | 8.42  | 1.90       | 1   | BOTTOM SLAB    | 7.82                                | •            |
|        | JCK   | TNAGT5A                                 | 45.000         |                      | 2.07                 | 93.15      | 1.40                              | 2.55       | 1    | BOTTOM SLAB   | 8.42  | 2.07       | 1   | BOTTOM SLAB    | 7.82                                |              |
|        | TRL   | TNAGT5B                                 | 45.000         | •                    | 2.07                 | 93.15      | 1.40                              | 2.55       | 1    | BOTTOM SLAB   | 8.42  | 2.07       | 1   | BOTTOM SLAB    | 7.82                                | •            |

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ASSEMBLED BY : RWW CHECKED BY : HLW



DOC

# LOAD FACTORS:

| DESIGN LOAD | RATING        | FACTORS       |
|-------------|---------------|---------------|
| LOAD TYPE   | MAX<br>FACTOR | MIN<br>FACTOR |
| DC          | 1.25          | 0.90          |
| DW          | 1.50          | 0.65          |
| ΕV          | 1.30          | 0.90          |
| EH          | 1.35          | 0.90          |
| ES          | 1.35          | 0.90          |
| LS          | 1.75          |               |
| WA          | 1.00          |               |

## NOTE:

RATING FACTORS ARE BASED ON THE STRENGTH I LIMIT STATE.

| $\bigcap$ | $\bigcirc$ | М     | М     | F | N   | Т | S : |
|-----------|------------|-------|-------|---|-----|---|-----|
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- 1.
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(#) CONTROLLING LOAD RATING 1 design load rating (hl-93) 2 DESIGN LOAD RATING (HS-20) 3 LEGAL LOAD RATING \*\* \*\* SEE CHART FOR VEHICLE TYPE

|   | PROJECT NO. <u>178P.14.R.142</u>  |                               |  |  |  |  |
|---|---|-------------------------------|--|--|--|--|
|   | CO  | UNTY                          |  |  |  |  |
|   | STATION: <u>13+47.70 -L</u>   |                               |  |  |  |  |
|   | SHEET 10 OF 10  |                               |  |  |  |  |
| DOCUMENT NOT CONSIDERED<br>FINAL UNLESS ALL<br>SIGNATURES COMPLETED | STATE OF NORTH CAROLINA<br>DEPARTMENT OF TRANSPORTATION<br>RALEIGH  |                               |  |  |  |  |
|   | STANDARD  |                               |  |  |  |  |
| NUMERAN CAROLINA  | LRFR SUMMARY F<br>REINFORCED CONCF<br>BOX CULVERTS<br>(NON-INTERSTATE TRAFI   | OR<br>Rete<br>Fic)            |  |  |  |  |
| E CONTRECTOR  | REVISIONS   | SHEET NO.                     |  |  |  |  |
| HardyWillis""4/28/2017<br>CC287FCF0223461                           | NO.         BY:         DATE:         NO.         BY:         DATE:           1         3 | C-10<br>total<br>sheets<br>10 |  |  |  |  |
|   | STD. NO. LRFR5  |                               |  |  |  |  |

#### DESIGN DATA:

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| SPECIFICATIONS                                  | A.A.S.H.T.O. (CURRENT) |  |  |  |  |  |
|---|------------------------|--|--|--|--|--|
| LIVE LOAD                                       | SEE PLANS              |  |  |  |  |  |
| IMPACT ALLOWANCE                                | SEE A.A.S.H.T.O.       |  |  |  |  |  |
| STRESS IN EXTREME FIBER OF                      |                        |  |  |  |  |  |
| STRUCTURAL STEEL - AASHTO M270 GRADE 36 -       | 20,000 LBS.PER SQ.IN.  |  |  |  |  |  |
| - AASHTO M270 GRADE 50W -                       | 27,000 LBS.PER SQ.IN.  |  |  |  |  |  |
| - AASHTO M270 GRADE 50 -                        | 27,000 LBS.PER SQ.IN.  |  |  |  |  |  |
| REINFORCING STEEL IN TENSION                    |                        |  |  |  |  |  |
| GRADE 60  | 24,000 LBS.PER SQ.IN.  |  |  |  |  |  |
| CONCRETE IN COMPRESSION                         | 1,200 LBS.PER SQ.IN.   |  |  |  |  |  |
| CONCRETE IN SHEAR                               | SEE A.A.S.H.T.O.       |  |  |  |  |  |
| STRUCTURAL TIMBER - TREATED OR                  |                        |  |  |  |  |  |
| UNTREATED - EXTREME FIBER STRESS                | 1,800 LBS.PER SQ.IN.   |  |  |  |  |  |
| COMPRESSION PERPENDICULAR TO GRAIN<br>OF TIMBER | 375 LBS.PER SQ.IN.     |  |  |  |  |  |
| EQUIVALENT FLUID PRESSURE OF EARTH              | 30 LBS.PER CU.FT.      |  |  |  |  |  |
|   | (MINIMUM)              |  |  |  |  |  |

## MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2012 ``STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

## CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

#### CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED 3/4" WITH THE FOLLOWING EXCEPTIONS: TOP CORNERS OF CURBS MAY BE ROUNDED TO 1-1/2"RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4"FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

## DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS. SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

# STANDARD NOTES

## ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE. ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION. VERTICAL CURVE ORDINATE. AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

## REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS. WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE

INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES, THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

## STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE 7/8" Ø SHEAR STUDS FOR THE  $3_4'' arnothing$  studs specified on the plans. This substitution shall be made at THE RATE OF 3 - 7/8"Ø STUDS FOR 4 - 3/4"Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF 7/8" Ø STUDS ALONG THE BEAM AS SHOWN FOR 3/4" Ø STUDS BASED ON THE RATIO OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST 5/16" IN THICKNESS AND DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY 1/16 INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

## HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB. UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB. METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY. IN CASE OF DISCREPANCY. THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.



STD. NO. SN